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Level 2 Strategic Flood Risk Assessment

Site HEA217

Prepared for: Southend-on-Sea City Council

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1 Background

This is a Level 2 Strategic Flood Risk Assessment (SFRA) site screening report for Site HEA217. The content of this Level 2 SFRA site screening report assumes the reader has already consulted the Southend-on-Sea City Council Level 1 SFRA and read the Southend-on-Sea City Council Level 2 SFRA Main Report and is therefore familiar with the terminology used in this report.

1.1 Site details

- Location: Immediately north of Prittlewell Chase, adjoining Priory House, within the Prittlewell inner city area of Southend-on-Sea.
- Site area: 0.2ha
- Existing site use: Brownfield
- Proposed site use: Residential

1.2 Topography

Figure 1-1 shows the topography of the site represented by the Environment Agency (EA) 1m resolution LiDAR. The site is mostly level, with only minor variation in the terrain. Slightly higher ground occurs in the central northern area, reaching a maximum elevation of 17.6mAOD. Towards the southern boundary, ground levels fall to around 15.6mAOD, where Prittle Brook runs eastward outside of the site.

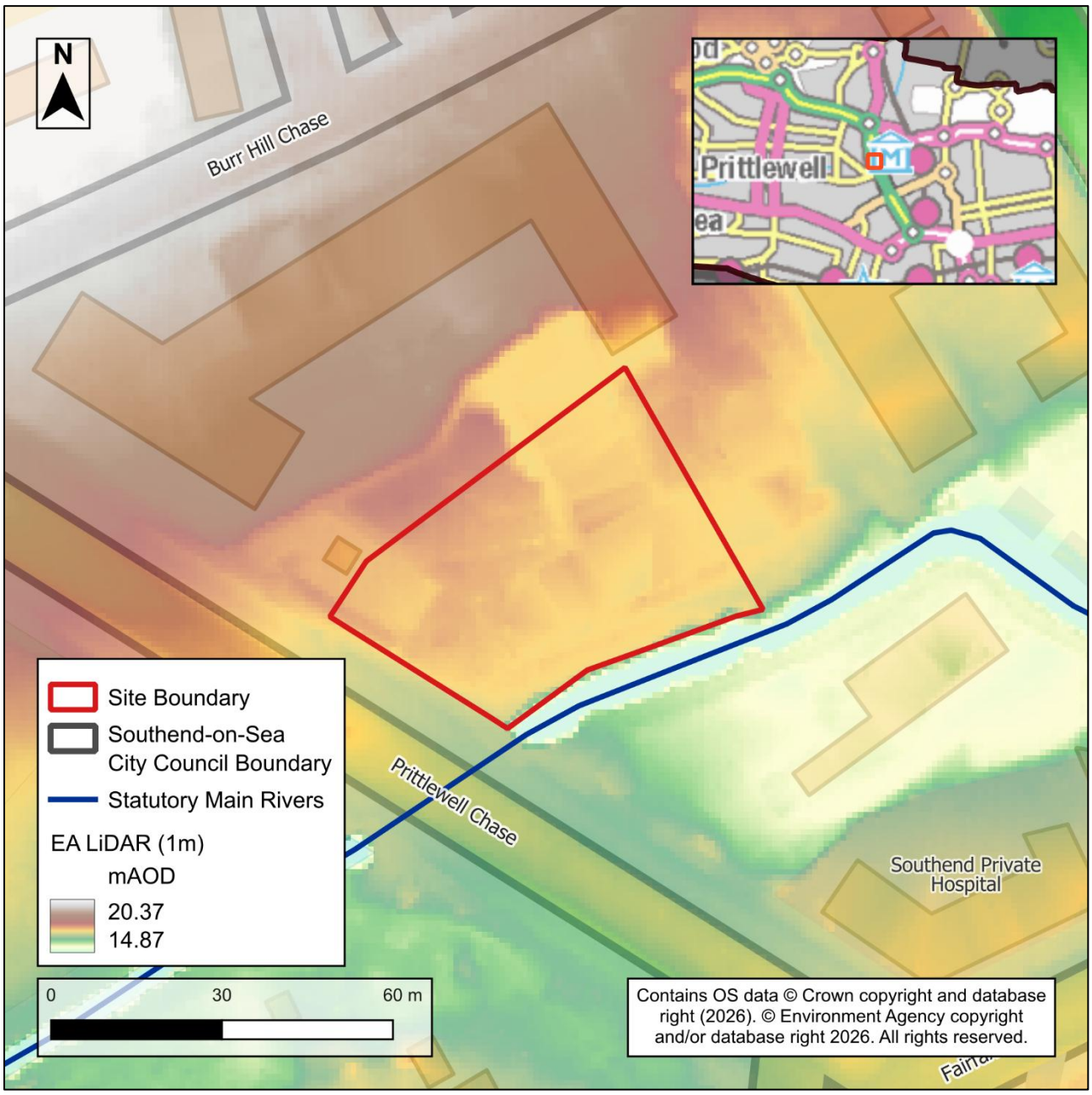


Figure 1-1: Topography of the site.

1.3 Geology and soils

Geology at the site consists of:

- Bedrock - London Clay Formation, comprised of clay, silt, and sand.
- Superficial - Head, consisting of clay, silt, sand, and gravel, is found in the north of the site. The southern portion of the site is underlain by River Terrace Deposits, comprised of sand and gravel.

Soils at the site consist of freely draining slightly acid loamy soils.

The likely impact of the geology and soils on drainage within the site is discussed within Section 5.

2 Sources of flood risk

2.1 Location of site within the catchment

The site is located in the catchment of Prittle Brook, a tributary of the tidal River Roach, within the upper reaches of the Crouch and Roach Operational Catchment. Prittle Brook flows in a northeasterly direction outside of the site, parallel to the southern boundary. This watercourse originates approximately 7km northwest of the site in West Wood in the north of Hadleigh, situated within Castle Point Borough. Upstream of the site, the catchment is predominantly urban, with the exception of allotment spaces, Belfairs Nature Reserve and Golf Course, as well as West Wood.

2.2 Existing drainage features

Based on online mapping, there are no existing drainage features within the site. However, as the existing land use is brownfield, there is likely to be existing drainage infrastructure present within the site.

Immediately outside of the site Prittle Brook, a statutory main river, flows eastward parallel to the southern boundary.

2.3 Fluvial and tidal

2.3.1 EA's Flood Map for Planning

Based on the EA's Flood Map for Planning (FMfP) Flood Zones (accessed 21 April 2026) and Flood Zone 3b (functional floodplain), defined in the Southend-on-Sea Level 1 SFRA, the percentage areas of the site within each Flood Zone are stated in

Table 2-1 and can be viewed in Figure 2-1. The FMfP does not consider the benefits from flood defence infrastructure.

The FMfP shows that 'rivers' are the source of flooding to the site, from Prittle Brook. The majority of the site (98%) is not at risk of fluvial flooding. However, the southern boundary of the site is within Flood Zones 3b, 3a, and 2. However, while the flood mapping appears to extend slightly onto the site, this is likely due to the resolution of the model and the delineation of the watercourse along the site boundary. The flooding appears to remain within the watercourse channel and is not indicative of out-of-bank flooding affecting the site.

Table 2-1: Present day flood risk based on the EA's FMfP.

Flood Zones	Flood Zone 3b (%)	Flood Zone 3a (%)	Flood Zone 2 (%)	Flood Zone 1 (%)
Percentage of site at risk* (%)	2	2	2	98

**This shows the percentage of the site at flood risk from a particular flood zone or event, including the percentage of the site at flood risk at a higher risk zone, e.g. Flood Zone 2 includes the Flood Zone 3a and 3b percentage. Flood Zone 1 + Flood Zone 2 = 100%.*

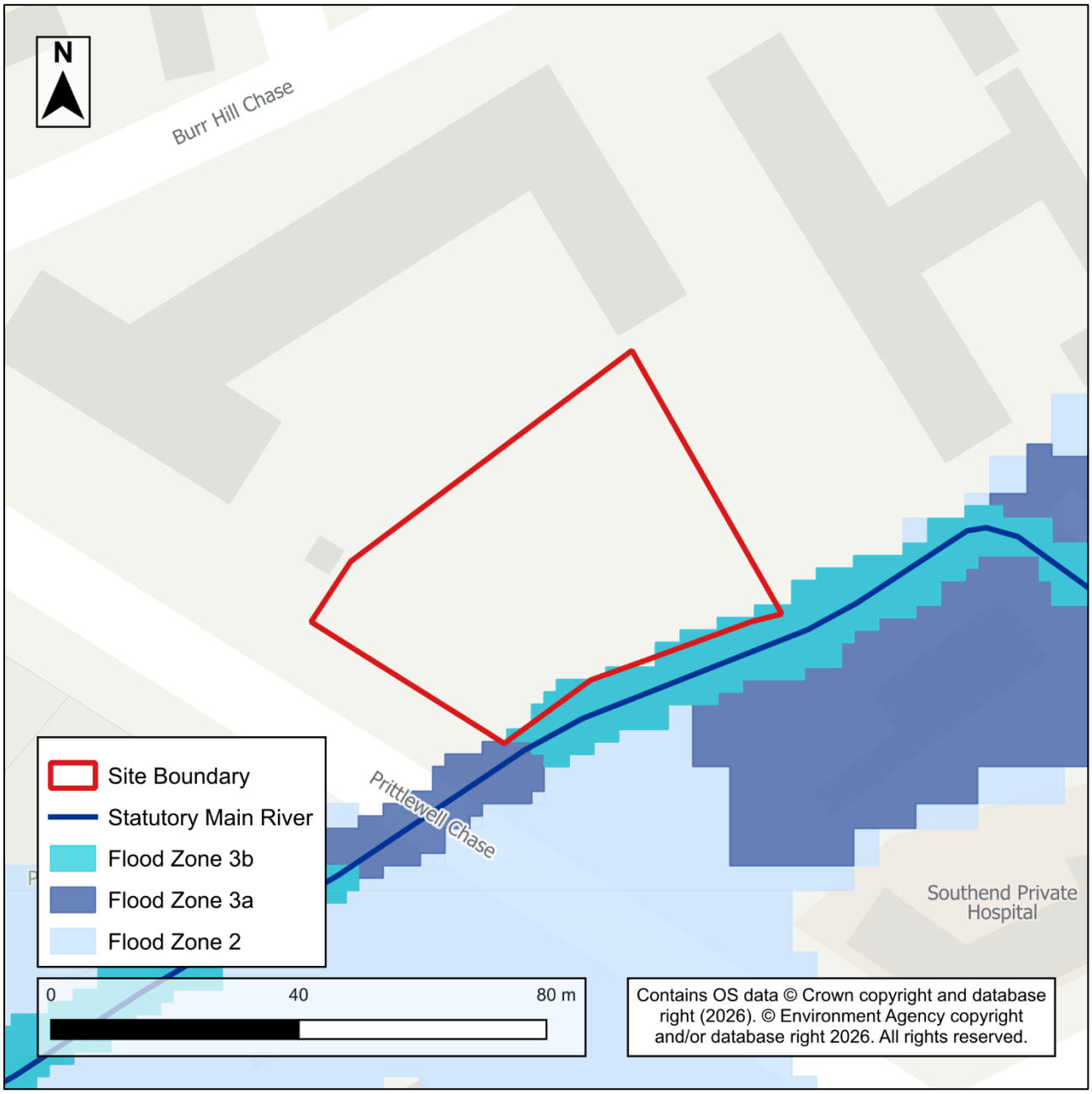


Figure 2-1: Present day fluvial flood risk, based on the EA FMfP and Rivers and Sea 3.3% AEP defended extent.

2.3.2 Fluvial - present day

The EA's 2016 Prittle Brook detailed hydraulic model has been used within this assessment. This model has been incorporated into the EA's FMfP. Depth and velocity hazard information is available for this modelling. Hazard outputs are not currently available for the 1% AEP and 0.1% AEP undefended events. It should be noted that the hydrology for this model was completed in 2016 and may not be appropriate for site-specific flood risk assessment, although in agreement with the EA this has informed the SFRA. For a site-specific FRA, the developer should undertake a review of the model hydrology compared with hydrological estimates based on latest data and methodologies. Should the hydrological inputs to the model be found to be under representative of current estimates, updates to the modelling with new hydrology may be required.

Detailed modelling suggests that flood outlines do not extend into the site. However, it should be noted that modelled outputs in this location are 2D only, meaning they only show out-of-bank flooding. The channel will remain wet in normal and flood conditions, with the modelling suggesting Flood Zone 3b remains in channel. Flood Zone 3a and 2 overtop the east bank, but not on the west bank into the site.

Depths for the 1% and 0.1% modelled event are shown in Figure 2-2 and Figure 2-4. Velocities for the 1% and 0.1% modelled event are shown in Figure 2-3 and Figure 2-5.

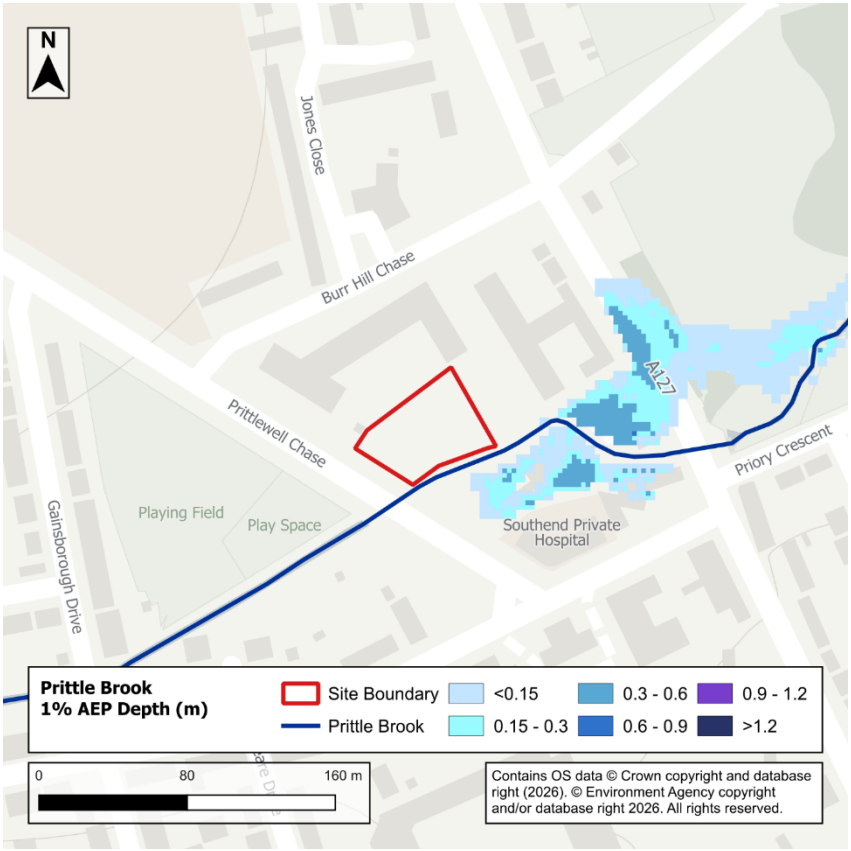


Figure 2-2: Prittle Brook 1% AEP depth across the site.

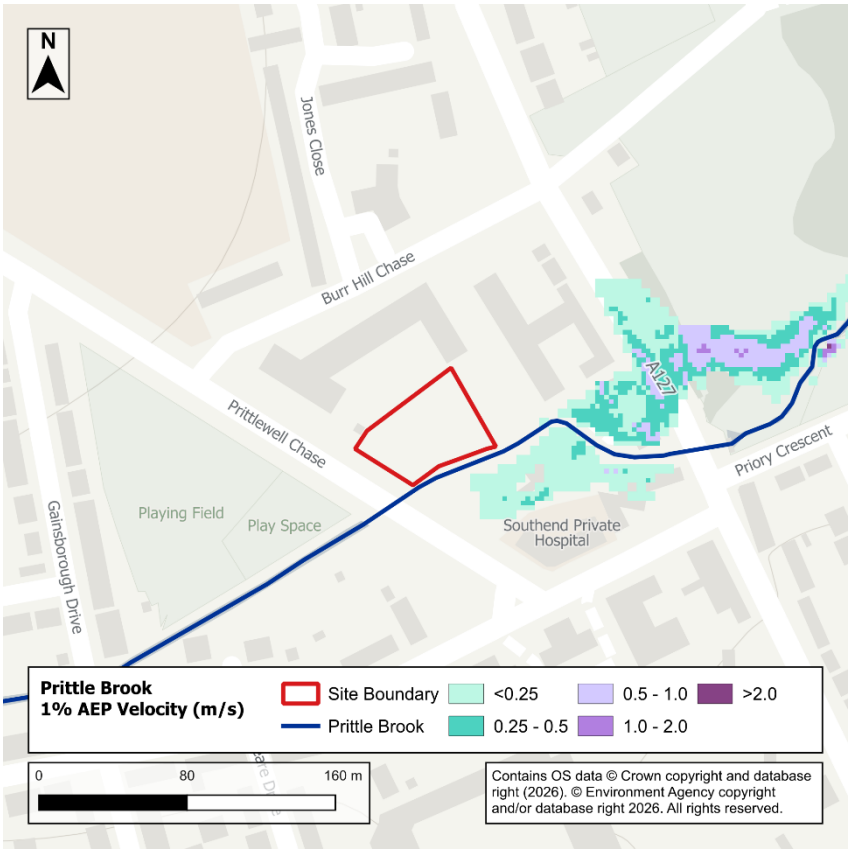


Figure 2-3: Prittle Brook 1% AEP velocity across the site.

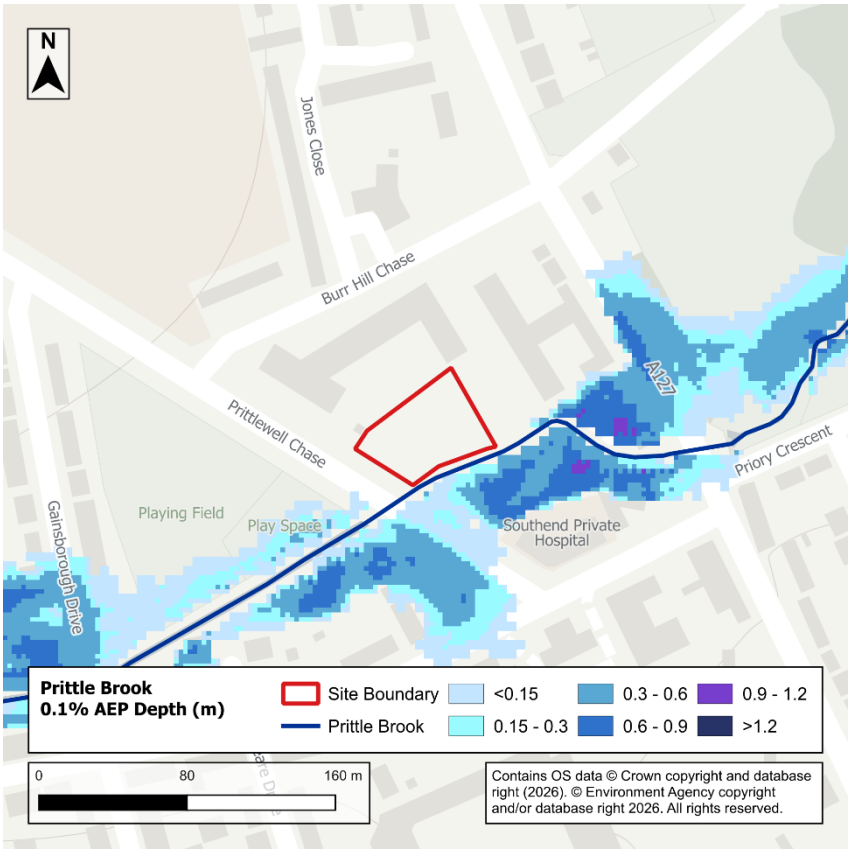


Figure 2-4: Prittle Brook 0.1% AEP depth across the site.

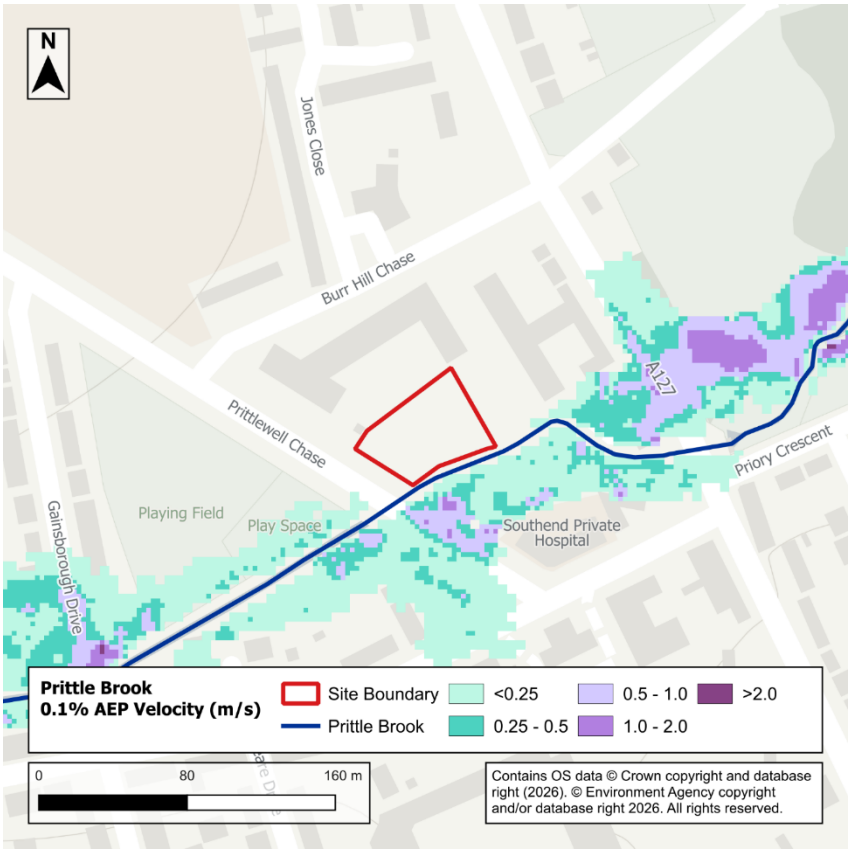


Figure 2-5: Prittle Brook 0.1% AEP velocity across the site.

2.3.3 Fluvial - climate change

The EA's 2016 Prittle Brook detailed hydraulic model has been used within this assessment. Data was available to assess the 1% AEP and 0.1% AEP plus 25% climate change events (Central 2080s) allowance. Depth, velocity, and hazard information is available for these datasets, and are mapped in Figure 2-6.

The EA Rivers and Sea 3.3% AEP has been used to represent future Flood Zone 3b. There is currently no available depth, hazard, or velocity information available for this dataset.

The available modelling suggests that the 3.3% and 1% AEP climate change events remain in channel along the left bank adjacent to the southern site boundary.

In the 0.1% AEP climate change event, the watercourse is shown to overtop just upstream of the site, and the encroaches into the western side of the site. Maximum depth, velocity, and hazard are low at 0.2m, 0.05m/s, and 'Danger for Some' respectively.

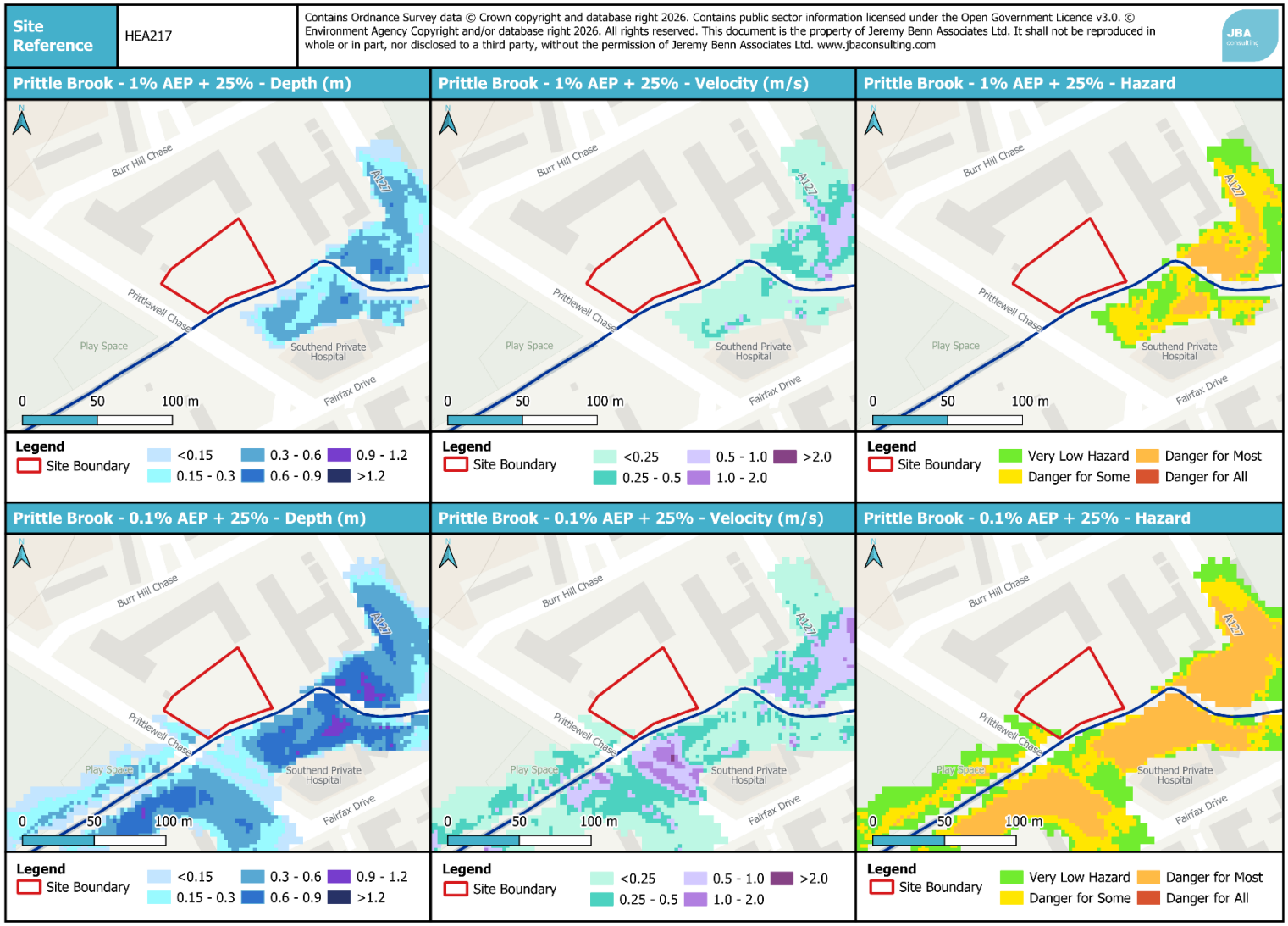


Figure 2-6: Prittle Brook model depth, velocity, and hazard for the 1% and 0.1% AEP plus 25% climate change events.

2.3.4 Tidal

As part of this SFRA, the existing EA hydraulic models of Southend and the Crouch and Roach estuary developed as part of the 2014 Essex, Suffolk and Norfolk Coastal modelling project were combined to cover the study area and have been used to assess the tidal risk across the study area. The combined tidal hydraulic model created as part of this SFRA was uplifted for the 2126 epoch for the higher central and upper end climate change scenarios.

The site is not shown to be at risk from tidal flooding in the present day or in future, according to the FMfP and local flood modelling.

2.4 Surface water

2.4.1 Available data

The EA's FMfP Surface Water Spatial Planning Dataset, released in May 2026, has been used within this assessment to inform the extent and depth of the present day and future surface water risk. Climate change outputs are available for the 3.3%, 1%, and 0.1% AEP events with the Upper End climate change allowance for the 2070s epoch.

The Risk of Flooding from Surface Water (RoFSW) map has been used within this assessment to inform the velocity and hazard of the present day surface water risk. There is currently no available velocity or hazard information to use to assess future surface water risk.

Descriptions refer to the maximum depth, velocity, and hazard values experienced in each area of the site within a given event

2.4.2 Present day

Figure 2-7 shows the surface water flood risk at the site. Figure 2-8 shows the depth, velocity, and hazard across the site in each event.

In the 3.3% AEP event, there is an area of isolated surface water ponding that encroaches into two places in the north of the site. As shown in Table 2-2, this covers 2% of the site. Maximum depths in this area of ponding are between 0.15m and 0.30m, and the entire area of risk has velocities that do not exceed 0.25m/s. The northernmost area of risk has a maximum hazard classification of 'Danger for Some', but the remaining area is a 'Very Low Hazard'.

In the 1% AEP event, the percentage of the site at risk increases to 10%, which includes a new area of risk along the northeastern boundary. As in the 3.3% AEP event, the northernmost area of risk displays the greatest depths, which are between 0.30m and 0.60m. The rest of the ponding has depths between 0.15m and 0.30m, with some areas remaining less than 0.15m. In one small area the maximum velocity exceeds 0.25m/s but does not exceed 0.50m/s, but the remainder of the ponding has velocities that do not

exceed 0.25m/s. The northernmost area of risk has a maximum hazard classification of 'Danger for Most', but the rest is a 'Very Low Hazard'.

The area of ponding increases most significantly in the 0.1% AEP event, to cover 47% of the site. Most of the northeast of the site is at risk, also extending southwards into the centre of the site. The maximum depths are between 0.30m and 0.60m at several points in the northeast, as well as by the eastern boundary. However, most of the depths are between 0.15m and 0.3m. There are two small areas where the maximum velocity exceeds 0.25m/s but does not exceed 0.5m/s, but the remainder of the ponding has velocities that do not exceed 0.25m/s. The northernmost area of risk has a maximum hazard classification of 'Danger for Most', but there are also parts of the site that are considered 'Danger for Some'. Just as in the 3.3% and 1% AEP events, most of the area of ponding has a 'Very Low Hazard' classification.

Table 2-2: Present day surface water flood risk based on the EA's FMfP Surface Water Spatial Planning dataset (extent, depth) and RoFSW dataset (velocity, hazard).

Event	3.3% AEP	1% AEP	0.1% AEP
Percentage of site at risk* (%)	2	10	47
Maximum depth (m)	0.15 - 0.30	0.30 - 0.60	0.30 - 0.60
Maximum velocity (m/s)	<0.25	0.25 - 0.50	0.25 - 0.50
Maximum hazard classification	Danger for Some	Danger for Most	Danger for Most

** The percentage surface water extents quoted show the percentage of the site at surface water risk from that particular event, including the percentage of the site at flood risk at a higher risk zone (e.g. the 1% AEP includes the 3.3% AEP percentage).*

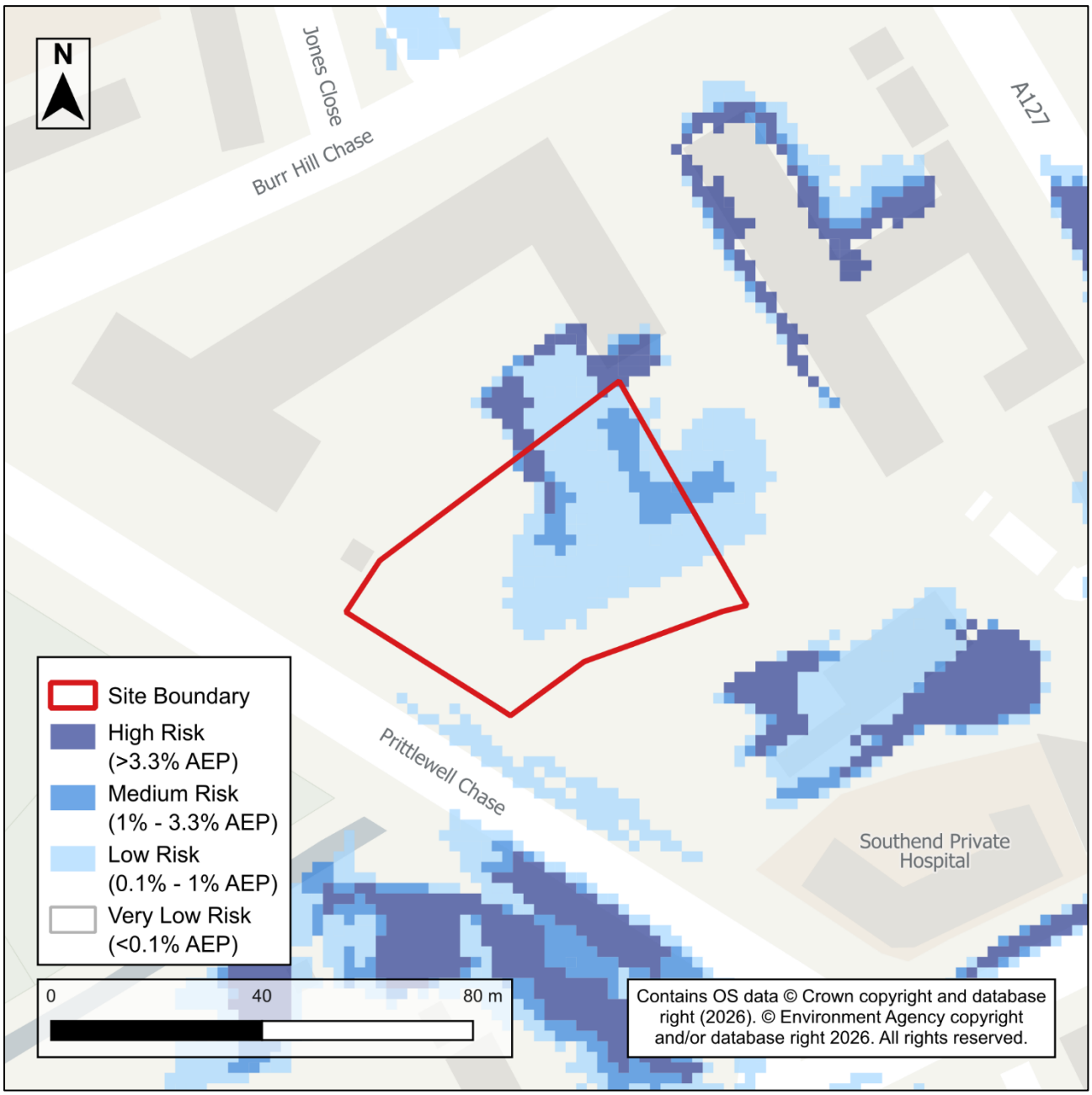


Figure 2-7: Present day surface water extents from the FMfP Surface Water Spatial Planning Dataset.

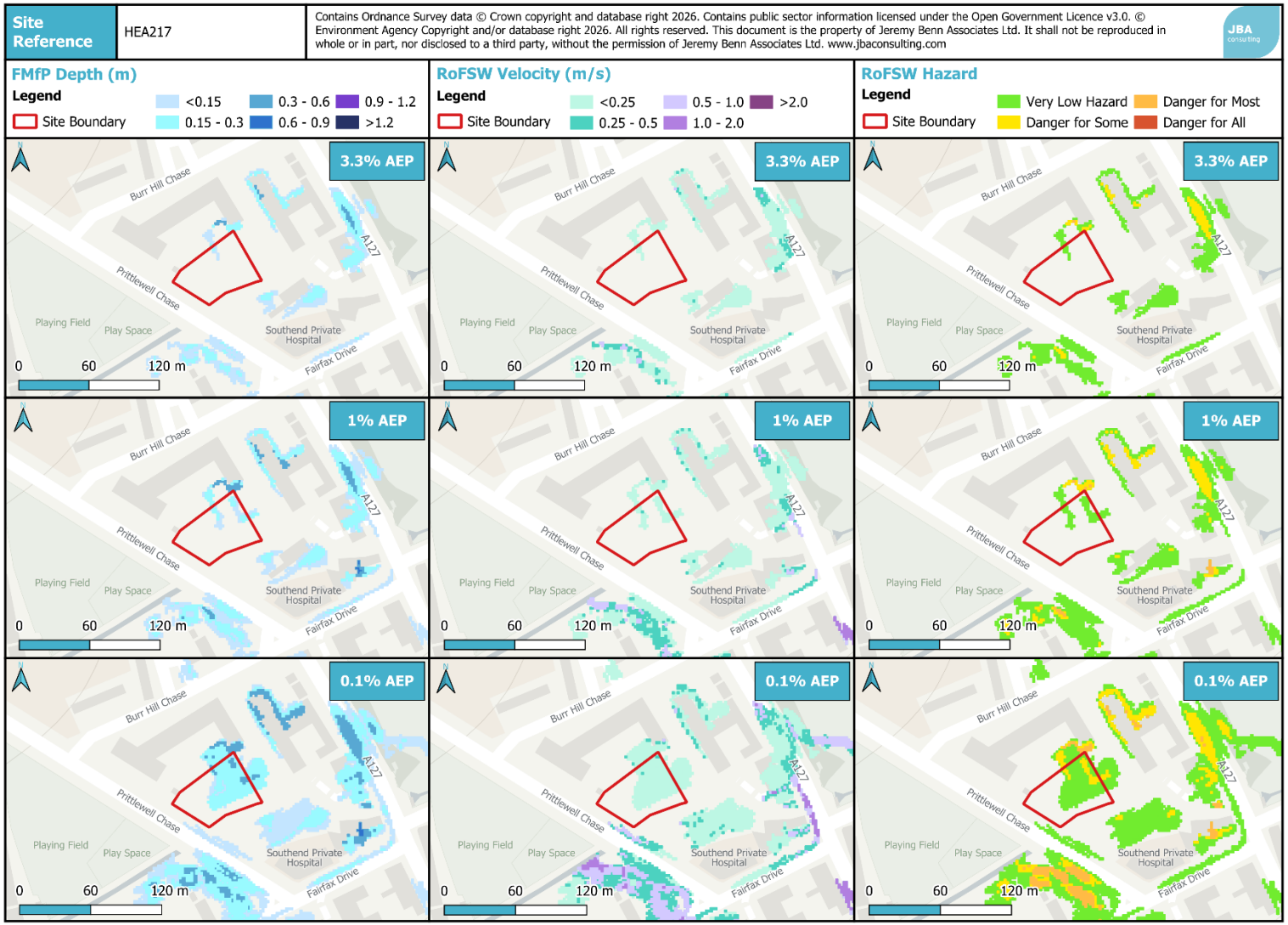


Figure 2-8: Present day surface water risk in the 3.3%, 1%, and 0.1% AEP events.

2.4.3 Climate change - 2070s Upper End

The extents and depths of surface water risk in the 3.3%, 1%, and 0.1% AEP Upper End climate change scenarios are shown in Figure 2-9

In the 3.3% AEP climate change event, the percentage of the site at risk is 8% greater than in the present day scenario. The extents are the same within the site as that of the present day 1% AEP event. Maximum depths increase from between 0.15m and 0.30m in the present day to between 0.30m and 0.60m in the climate change event, in the northernmost corner of the site.

In the 1% AEP climate change event, the area of the site at risk increases significantly from 10% to 45%, as shown in Table 2-3. This covers most of the east of the site, excluding an area in the southeast. The maximum depths remain between 0.30m to 0.06m, but these depths are now also found in an area by the eastern boundary.

In the 0.1% AEP climate change event, the percentage of site at risk increases by 6% compared to the present day, with a new area of pooling in the northwest of the site. As in the other events, the maximum depths remain between 0.30m to 0.60m. The maximum depths in the new area of ponding in the northwest are between 0.15m and 0.30m.

Table 2-3: Comparison of surface water flood risk between the present day and climate change scenarios based on the EA's FMfP Surface Water Spatial Planning.

Event	1% AEP	1% AEP plus 2070s Upper End Climate Change
Percentage of sites at risk* (%)	10	45
Maximum depth (m)	0.30 - 0.60	0.30 - 0.60

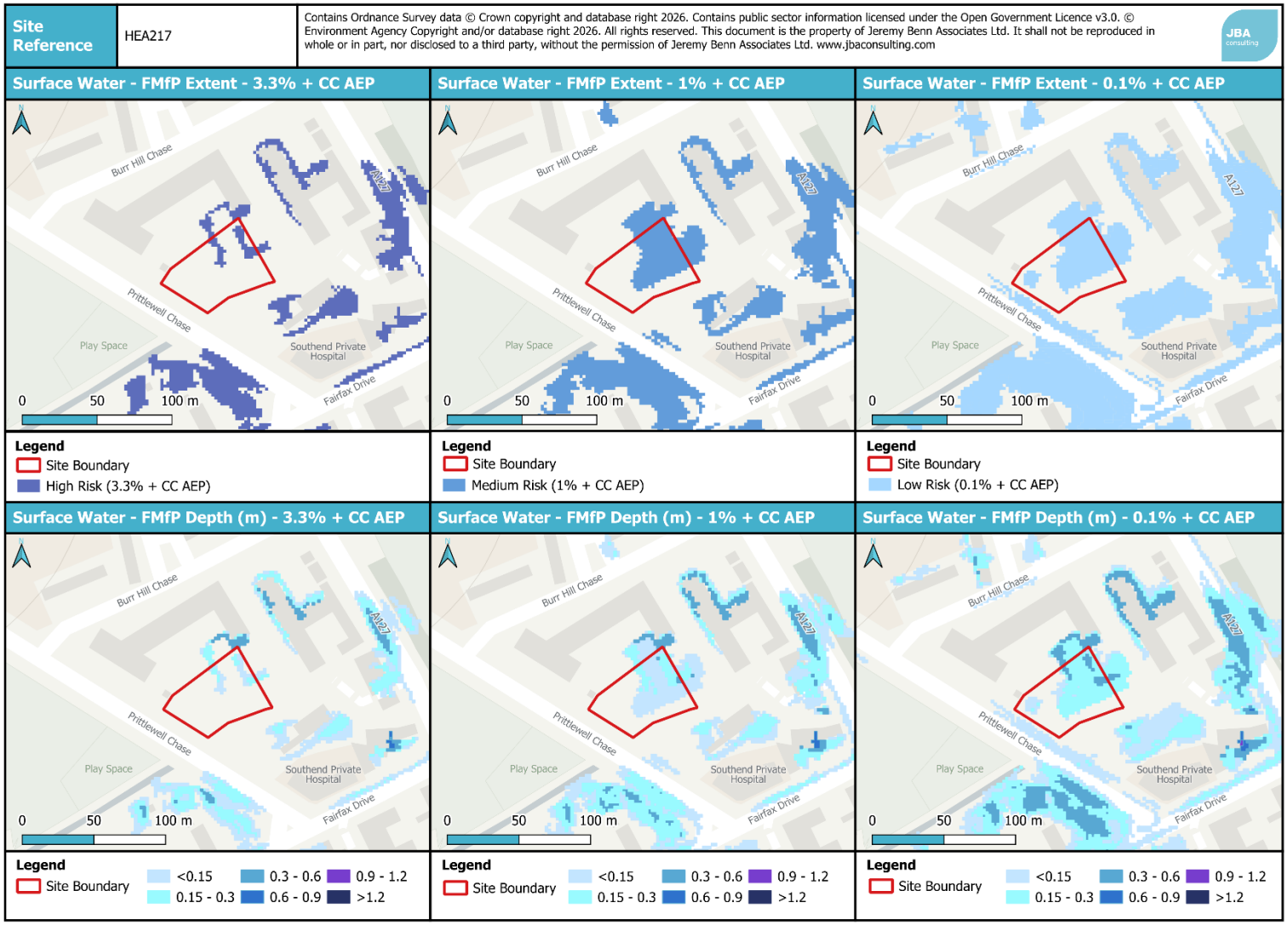


Figure 2-9: Extents and depths of surface water risk in the 3.3%, 1%, and 0.1% AEP Upper End climate change scenarios.

2.5 Groundwater

The JBA Groundwater Emergence Map, mapped in Figure 2-10, shows that the southern portion of the site has groundwater levels between 0.5m and 5m below the ground surface. This means that there is a risk of flooding to subsurface assets, but surface manifestation of groundwater is unlikely. The JBA Groundwater Emergence Map shows that the north of the site has a negligible risk from groundwater flooding due to the nature of the local geological deposits.

This assessment does not negate the requirement that an appropriate assessment of the groundwater regime should be carried out at the site-specific Flood Risk Assessment (FRA) stage.

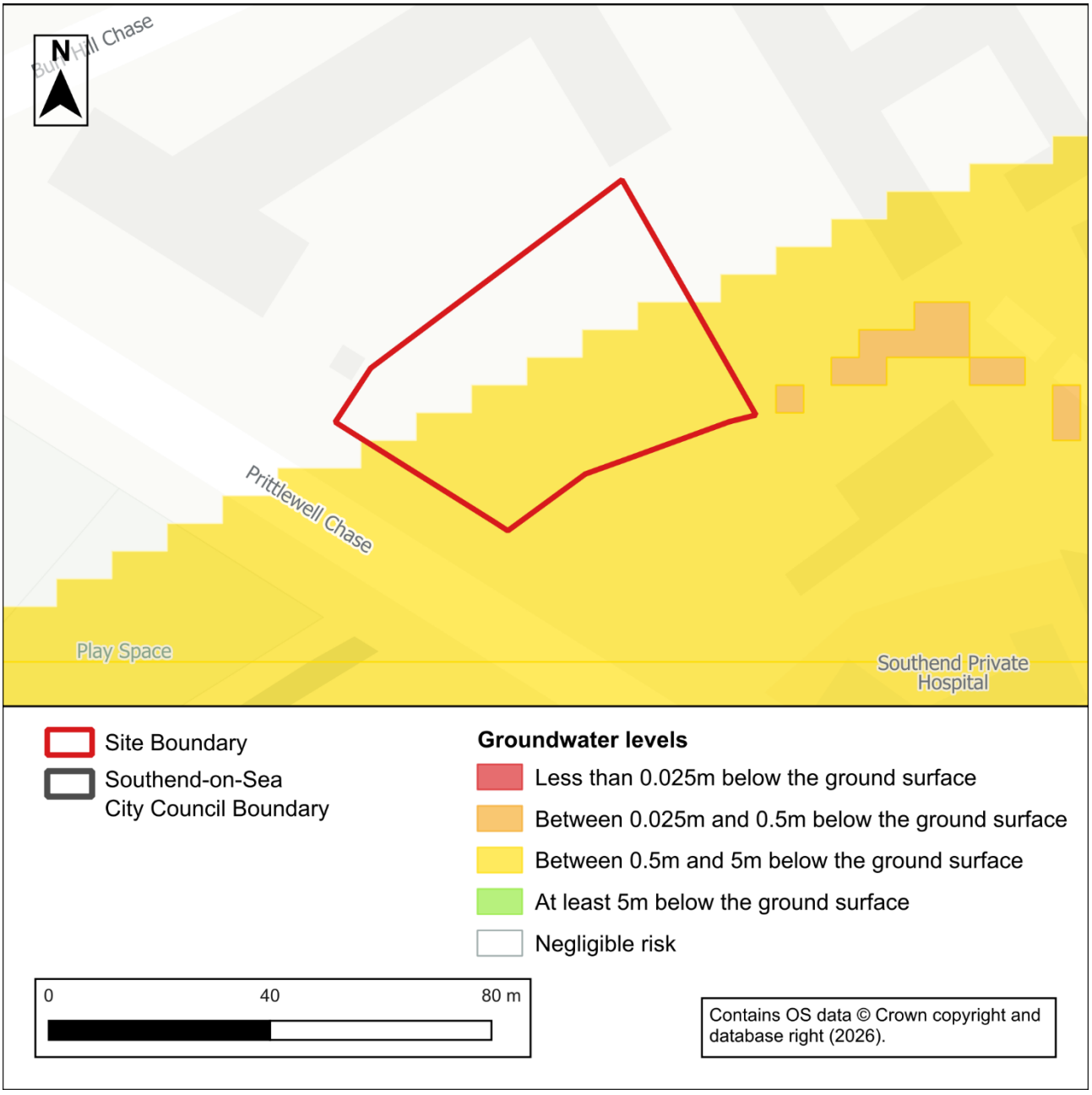


Figure 2-10: Groundwater levels across the site.

2.6 Sewers

Anglian Water provided a redacted GDPR compliant version of their DG5 dataset for the study area, which identifies internal and external historic sewer flooding records between 2022 and 2026.

There are no records of flooding at the site.

The available data should always be supplemented by direct consultation with Anglian Water to ascertain if there is any site-specific risk from a public sewer. This is because sewer flood risk information is not publicly available and would need to be considered on a site-specific basis.

2.7 Flood history

The EA's Historic Flood Map shows no records of flooding at the site.

A Section 19 flood investigation report from 24 August 2013 (democracy.southend.gov.uk), describes that Prittle Brook appeared to flood within Priory Park, with water backing up on Victoria Avenue. This road runs parallel to the eastern boundary of the site, approximately 100m east.

Data provided by Essex County Fire and Rescue Service shows no records of flooding at the site.

3 Flood risk management infrastructure

3.1 Defences

The EA AIMS dataset shows that the site is not protected by any formal flood defences.

3.2 Residual risk

Prittle Brook is culverted directly southwest of the site beneath Prittlewell Chase, as well as underneath Victoria Avenue around 100m southeast of the site. The modelling indicates that the culverts restrict flow, causing overtopping of the watercourse that predominantly affects the lower-lying land south of the site. There is a residual risk that culvert blockage could exacerbate the flood risk and result in water backing up onto the site.

4 Emergency planning

4.1 Flood warnings and alerts

The south of the site is located in the EA's 'The Eastwood Brook and Prittlewell Brook in the Southend area' (051FWFEF7B) Flood Warning Area and the EA's 'The River Roach, Prittle Brook and Eastwood Brook' (051WAFEF7B) Flood Alert Area.

4.2 Access and escape

Safe access and escape will need to be demonstrated in the 1% AEP plus climate change fluvial and surface water events. Site drainage proposals should address the requirements for access routes, avoid impeding surface water flows and preserve the storage of surface water to avoid exacerbation of flood risk elsewhere on the site and in the wider catchment.

4.2.1 Potential access

The site is accessible from the west via Prittlewell Chase. The road is a dual carriageway, where traffic is travelling one-way south past the site. To travel north along Prittlewell Chase requires continuing to Fairfax Drive and then turning right twice.

4.2.2 Fluvial

Access to the site via Prittlewell Chase is possible in all modelled events. However, as this road is a dual carriageway, there is fluvial flood risk when escaping the site to the south. Escape is likely possible in all events, but there is still fluvial risk on the escape roads that require consideration, particularly in the 0.1% AEP event.

All access and escape routes are clear in the 3.3% defended present day scenario (Flood Zone 3b). Detailed hydraulic modelling shows that that the Prittle Brook overtops in the undefended 1% AEP event (Flood Zone 3a) and the 1% AEP plus 25% climate change event, impacting travel north via Victoria Avenue (A127) with depths between 0.3m and 0.6m. All other routes are clear in these two events.

In the 0.1% AEP undefended event (Flood Zone 2), escape should be possible by travelling via Prittlewell Chase to Fairfax Drive, and either continuing west or heading east to Victoria Avenue (A127) before proceeding south. Travelling via these routes, there is fluvial flood risk on the road southwest of the site along Prittlewell Chase. If continuing west, there is also flood risk on the road west of junction from Prittlewell Chase on Fairfax Drive. However, as depths in these locations are below 0.3m, they may be passable with a vehicle. Escape north via either Prittlewell Chase or Victoria Avenue (A127) may be obstructed. Depths on Prittlewell Chase going north are between 0.3m and 0.6m, and on or Victoria Avenue (A127) maximum depths reach between 0.6m and 0.9m.

4.2.3 Surface water

Access and escape should be possible in all modelled present day events by travelling via Prittlewell Chase to Fairfax Drive, and south along Victoria Avenue (A127). Access and escape may be obstructed continuing north along Victoria Avenue (A127), as there is an area of pooling on the road with depths between 0.30m and 0.60m in all events, including the 1% AEP plus climate change scenario.

While most of Prittlewell Chase is clear from flood risk, if travelling north on this road there is an area of ponding in all events immediately after the turning from Fairfax Drive. Depths here are between 0.15m and 0.30m in the 3.3% AEP and 1% AEP events, which may be passable with a vehicle. However, in the 0.1% AEP event, this area of ponding has depths between 0.30m and 0.60m. This is also the case in the 1% AEP event plus climate change event, but in this scenario these depths do not cover the entire road.

In the 0.1% AEP and 1% AEP plus climate change events, there is an area of ponding along Prittlewell Chase directly southwest of the site. However, these depths are less than 0.15m. In the 1% AEP plus climate change surface water event, there is also a stretch of flood risk along Fairfax Drive, and these depths are between 0.15m and 0.30m.

4.3 Dry islands

The site is not located on a dry island.

5 Requirements for drainage control and impact mitigation

5.1 Broadscale assessment of possible SuDS

Surface water at the site should be discharged in accordance with the drainage hierarchy as set out in the National standards for SuDS (gov.uk). SuDS that provide opportunities to deliver multiple benefits including volume control, water quality, amenity, and biodiversity should be prioritised. SuDS proposals should be discussed with relevant stakeholders (Local Planning Authority (LPA), Lead Local Flood Authority (LLFA), and the EA) at an early stage to understand possible constraints.

Surface water discharge rates should not exceed pre-development discharge rates for the site and should be designed to be as close to greenfield runoff rates as reasonably practical in consultation with the LLFA.

The site is located within the Prittle Brook Critical Drainage Area (CDA). The CDAs currently designated were identified in the Southend-on-Sea SWMP 2015 (southend.gov.uk). It is particularly important that any development within these CDAs will need to demonstrate an adequate surface water drainage system which is maintainable for the lifetime of the development within a site specific FRA. Developers will need to provide details of the long term maintenance of the surface water drainage system. Developers should collaborate with the LLFA to identify all sources of flood risk, help reduce risk to existing communities, and explore grant funding opportunities for these wider improvements. Furthermore, growth allocations should take CDAs into account when assessing site suitability for development, ensuring that flood risk is properly managed and mitigated during planning.

Opportunities to incorporate source control techniques such as green roofs, permeable surfaces and rainwater harvesting must be considered in the design of the site.

The following factors should be considered when assessing whether infiltration is suitable at the site:

- Groundwater levels are indicated to be between 0.5 and 5m below ground level and there is a risk of flooding to subsurface assets and below ground development such as basements. Groundwater monitoring is recommended to determine the seasonal variability of groundwater levels, as this may affect the design of the surface water drainage system.
- BGS data indicates that the underlying geology is clay, silt, and sand, which is likely to have highly variable permeability. This should be confirmed through infiltration testing. Off-site discharge in accordance with the SuDS hierarchy may be required to discharge surface water runoff from the site.
- The site is not located within a Groundwater Source Protection Zone and there are no restrictions over the use of infiltration techniques with regard to groundwater quality.

- The site is not located within a historic landfill site.

If it is proposed to discharge runoff to a watercourse or sewer system, the condition and capacity or the receiving watercourse or asset should be confirmed through surveys and the discharge rate agreed with the asset owner.

5.2 Opportunities for wider sustainability benefits and integrated flood risk management

The following are opportunities for wider sustainability benefits and integrated flood risk management at the site:

- It may be possible to reduce site runoff by maximising the permeable surfaces on site using a combination of permeable surfacing and soft landscaping techniques.
- Opportunities to incorporate filtration techniques such as filter strips, filter drains and bioretention areas must be considered.
- Consideration should be made to the existing condition of Prittle Brook and its Water Framework Directive objectives for water quality. The use of multistage SuDS treatment will improve water quality of surface water runoff discharged from the site and reduce the impact on receiving water bodies and can also contribute towards requirements for Biodiversity Net Gain. Prittle Brook (GB105037028730) is considered to be of moderate ecological status.

6 NPPF and planning implications

6.1 Exception Test requirements

The Local Planning Authority will need to confirm that the Sequential Test has been carried out in line with national guidelines. The Sequential Test will need to be passed before the Exception Test is applied.

The NPPF classifies residential development as 'More Vulnerable'.

The Exception Test will be required for this site if 'More Vulnerable' development is proposed within Flood Zone 3a. 'More Vulnerable' development is not permitted in Flood Zone 3b.

6.2 Requirements and guidance for site-specific Flood Risk Assessment

At the planning application stage, a site-specific FRA will be required as the proposed development site is:

- within Flood Zones 2, 3a, and 3b
- in areas with critical drainage problems
- at risk of flooding from surface water
- identified as being at increased flood risk in the future.

All sources of flooding should be considered as part of a site-specific FRA.

Consultation with the LPA, LLFA, Anglian Water, and the EA should be undertaken at an early stage.

Any FRA should be carried out in line with latest guidance including the National Planning Policy Framework (NPPF), Flood Risk and Coastal Change Planning Practice Guidance (PPG), and Southend-on-Sea City Council Local Plan Policies.

The development should be designed with mitigation measures in place where required.

6.3 Guidance for site design and making development safe

Where possible, the site layout should adopt a sequential approach by steering more vulnerable aspects of the development outside of the southeast extent of the site which is at the highest risk of surface water flooding. Where this is not possible, given the small size of the site, Finished Floor Levels should be raised above the expected height of flooding in line with the EA's guidance and any raising of ground levels should ensure that flood risk is not increased elsewhere.

A buffer of at least 8 metres should be maintained between Prittle Brook and any built development.

The developer should undertake a review of the model hydrology for the Prittle Brook model compared with hydrological estimates based on latest data and methodologies. Should the

hydrological inputs to the model be found to be under representative of current estimates, updates to the modelling with new hydrology may be required.

The risk from surface water flow routes should be quantified as part of a site-specific FRA, including a drainage strategy, so runoff magnitudes from the development are not increased by development across any surface water flow routes. A drainage strategy should help inform site layout and design to ensure runoff rates are as close as possible to pre-development greenfield rates, with areas of surface water ponding used as open space and SuDS or water compatible/essential infrastructure uses only.

Arrangements for safe access and escape will need to be provided for the 1% AEP fluvial and surface events with an appropriate allowance for climate change, considering depth, velocity, and hazard. Design and access arrangements will need to incorporate measures, so development and occupants are safe.

Provisions for safe access and escape should not impact on surface water flow routes or contribute to loss of floodplain storage. Consideration should be given to the siting of access points with respect to areas of surface water flood risk.

7 Conclusions

Summary of the key areas of flood risk across the site:

- **Fluvial:** Prittle Brook flows eastward parallel to the southern boundary. The majority of the site (98%) is not at risk of fluvial flooding. The southern boundary of the site is within Flood Zones 3b, 3a, and 2. However, this flood risk remains confined to the channel along the western boundary, with overtopping into the southwest of the site occurring only during the 0.1% AEP climate change event.
- **Surface water:** There is an area of surface water ponding in the northeast of the site. This area of risk increases significantly in the 0.1% AEP event, covering 47% of the site, extending into the centre of the site from the east.
- **Groundwater:** The southern portion of the site has groundwater levels between 0.5m and 5m below the ground surface. Here there is a risk of flooding to subsurface assets, but surface manifestation of groundwater is unlikely.
- **Residual:** Prittle Brook is culverted directly southwest of the site beneath Prittlewell Chase, as well as underneath Victoria Avenue around 100m southeast of the site. In the event of a blockage, this could cause water to back up and overtop, potentially encroaching into the south of the site.

The following points should be considered in development of this site:

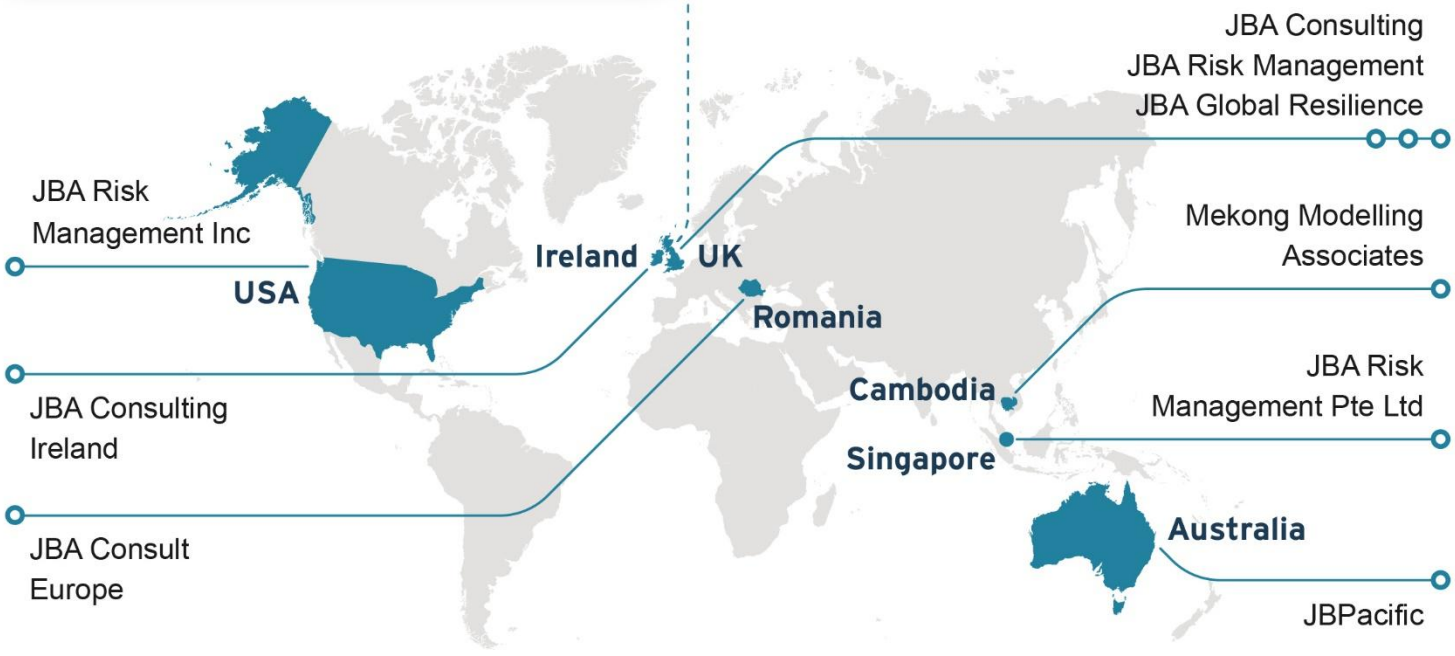
- The Exception Test will be required for this site if 'More Vulnerable' development is proposed within Flood Zone 3a.
- A site-specific FRA should be undertaken, which assesses all sources of flood risk.
- Safe access and escape routes should be demonstrated in the 1% AEP plus climate change surface water and fluvial events. This initial assessment has shown that access and escape will likely be possible, although there is flood risk on the access roads that requires consideration.
- The site layout must adopt a sequential approach. A buffer of at least 8 metres should be maintained between Prittle Brook and any built development, with more vulnerable aspects of the development steered away from the area of highest risk in the south of the site.
- A carefully considered and integrated flood resilient and sustainable drainage design should be put forward, including a site-specific Surface Water Drainage Strategy, and SuDS maintenance and management plan and supported by detailed modelling.
- Anglian Water should be consulted at an early stage on drainage proposals for the site to identify any potential constraints or existing sewer flooding issues that may not have been identified through this high level assessment.

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